

**The 6th International Conference on Hydraulic and Civil Engineering**

**ICHCE 2020**



# **F**ast **A**nalysis **S**ystem for **T**unneling

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**1 Background of the research**

**2 Solutions for the safe tunneling**

**3 Building the FAST platform**



**4 Verification of FAST**



## What is the safe design and construction In Tunneling



- **Is the tunneling safe in China ?**
- **What is the safe problem in tunneling ?**



## What is the safe design and construction

- Tunnel **design** is usually a **blind design**:
  - Prior **geological investigation** is often not enough;
  - Rock **parameters** is not accuracy;
  - Designers usually not **design bolts and shotcretes**.
- So, the tunnel **constructor** is often **blind too**.
- They construct tunnel mainly by **experience** not **science**.



## How to solving the problem



- Using **Rational (scientific) design** replace the **empirical design for tunnel**.

Why we can **not do the rational design** during tunneling even if we know (get parameters) of the rock of tunnel face after excavation ?





# 1 Background—Long Time

**Because we can**

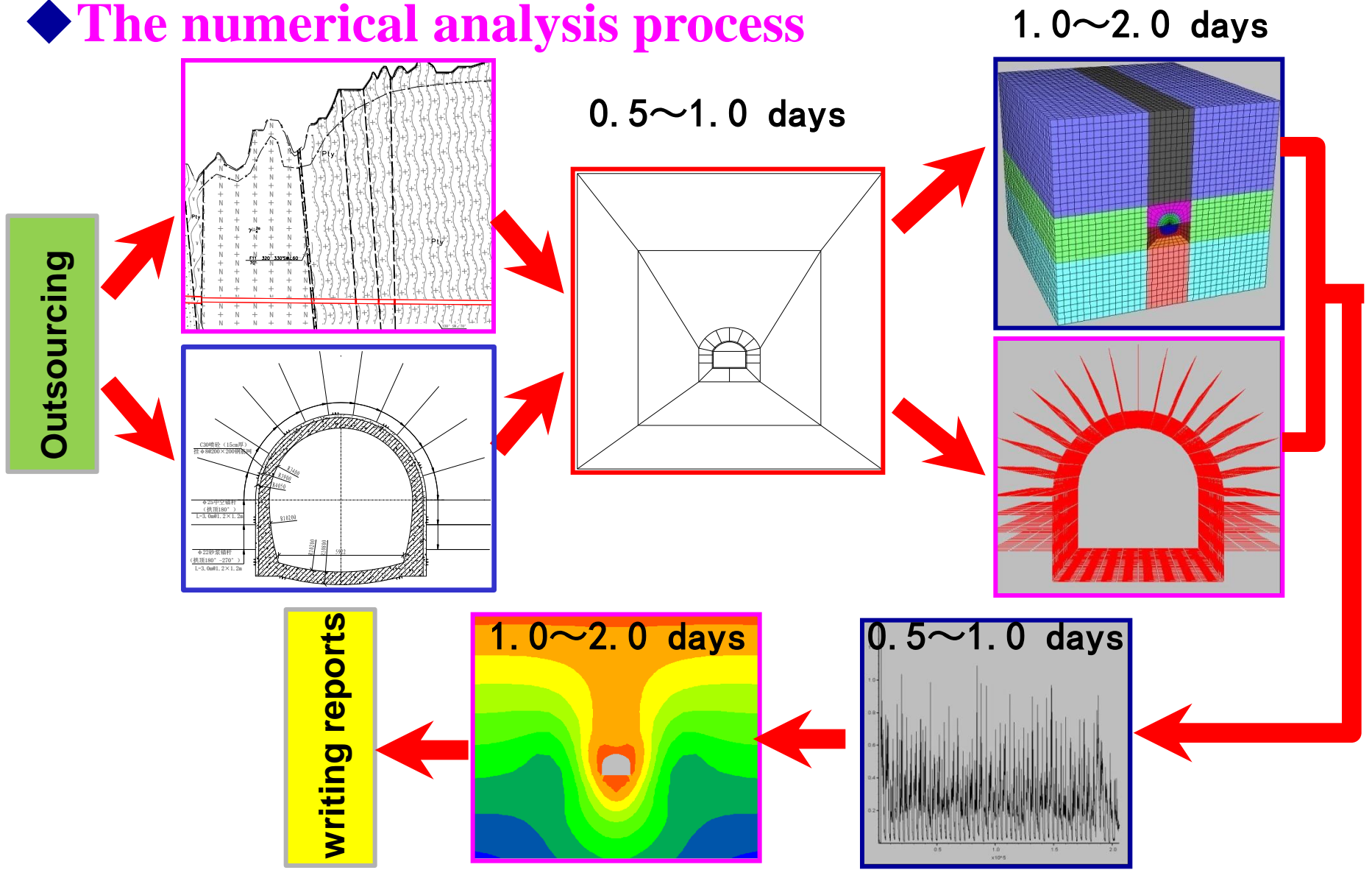
**not make the design calculation**

**so fast that to**

**follow the construction schedule.**

# 1 Background—Long Time

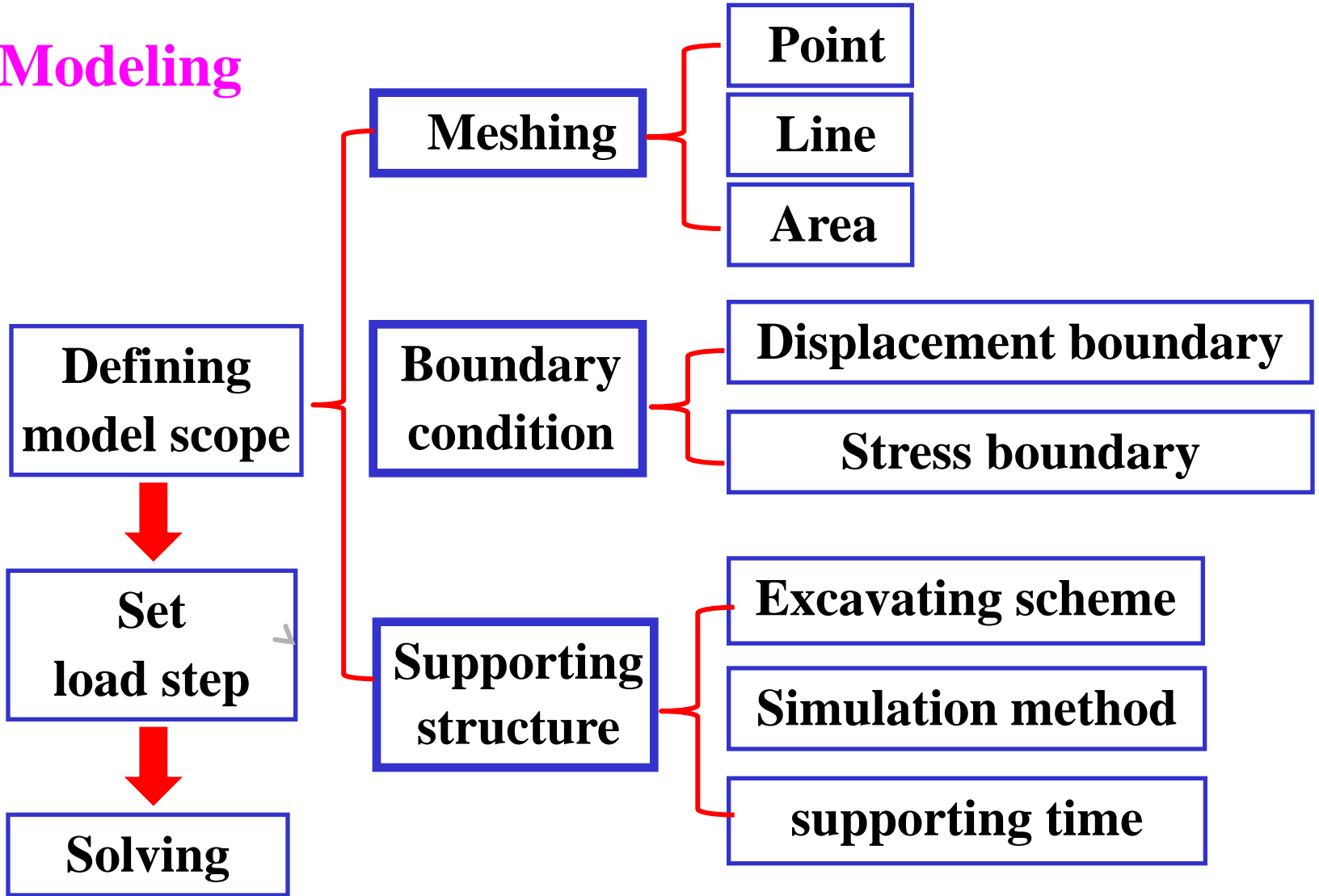
## ◆ The numerical analysis process





# 1 Background—Tedious numerical methods

## ◆ Modeling





Can we develop a system which have following features:

- **Fast**—from few days reduce to several minutes
- **Simple to use**—apply to ordinary engineers, not professional numerical experts.
- **Multi-functions :**
  - ✓ Consider **main geological structures of the rockmass**
  - ✓ Consider **excavation and support methods**
  - ✓ **Optimize the support (bolts and shotcret)**
  - ✓ **Quickly back analysis for mechanical parameters of the rockmass**
  - ✓ **Quickly evaluate the stability of surrounding rock and supports**





## 2 Solution for safe tunneling—Contradiction

### ◆ Requirement

**Multifunctions:** requiring the large FEM to simulate the particularity of geotechnical engineering (e.g. structural surface, construction measures)

**Fast and simple:** may not to start with meshing, not involve with elastoplastic/nonlinear solving methods, and the simulation of construction measures **for the users**

Contradiction



## 2 Solution for safe tunneling—Solving Ideas

### ➤ **Prior analysis and research**

- —**Find out the mechanical relations between the tunnel displacements and rock property, load and tunnel size ... ..**

### ➤ **Not so accuracy-but Fast and simple**

- —**No need to get hundreds or thousands of displacements of all nodes, only 20~30 important points along the tunnel face.**

- —**NO need to reach accuracy of 99.99% or 99.999% as most FEM, 80% accuracy is enough !!!**



## 2 Solutions for safe tunneling

### ➤ **Prior analysis**

Using the specialized analysis tool to **simulate various tunnels**

(different tunnel sizes, rock types, with different geological structures, under different depth and supporting measures)

Analyzing its displacements, stress and supporting forces. (11p,15y)

### ➤ **Build the relational database**

**Studying the correspondence** between displacements, stresses and main effect factors to **build the relational database.**



## 2 Solutions for safe tunneling

### ➤ **Form the samples**

Taking the relational database of the inputs and outputs of the systematic **numerical tests as main samples**, which is satisfy the stress equilibrium condition, deformation compatibility condition, Elastic-plastic criteria and the strength of supporting structures. And take the **experts experience, field monitoring data and related specifications as boundary samples**.

### ➤ **Build the automated analysis platform**

Using the powerful artificial **neuron network** method to **build the fast intelligence analysis system of tunnels** for the designers and constructors.



## Main influence factors of tunnel stability

- **the size of tunnel**
- **surrounding rock type ( $E, \nu, C, \varphi$ )**
- **the depth of tunnel**
- **faults or joints near the tunnel**
- **supporting structures**



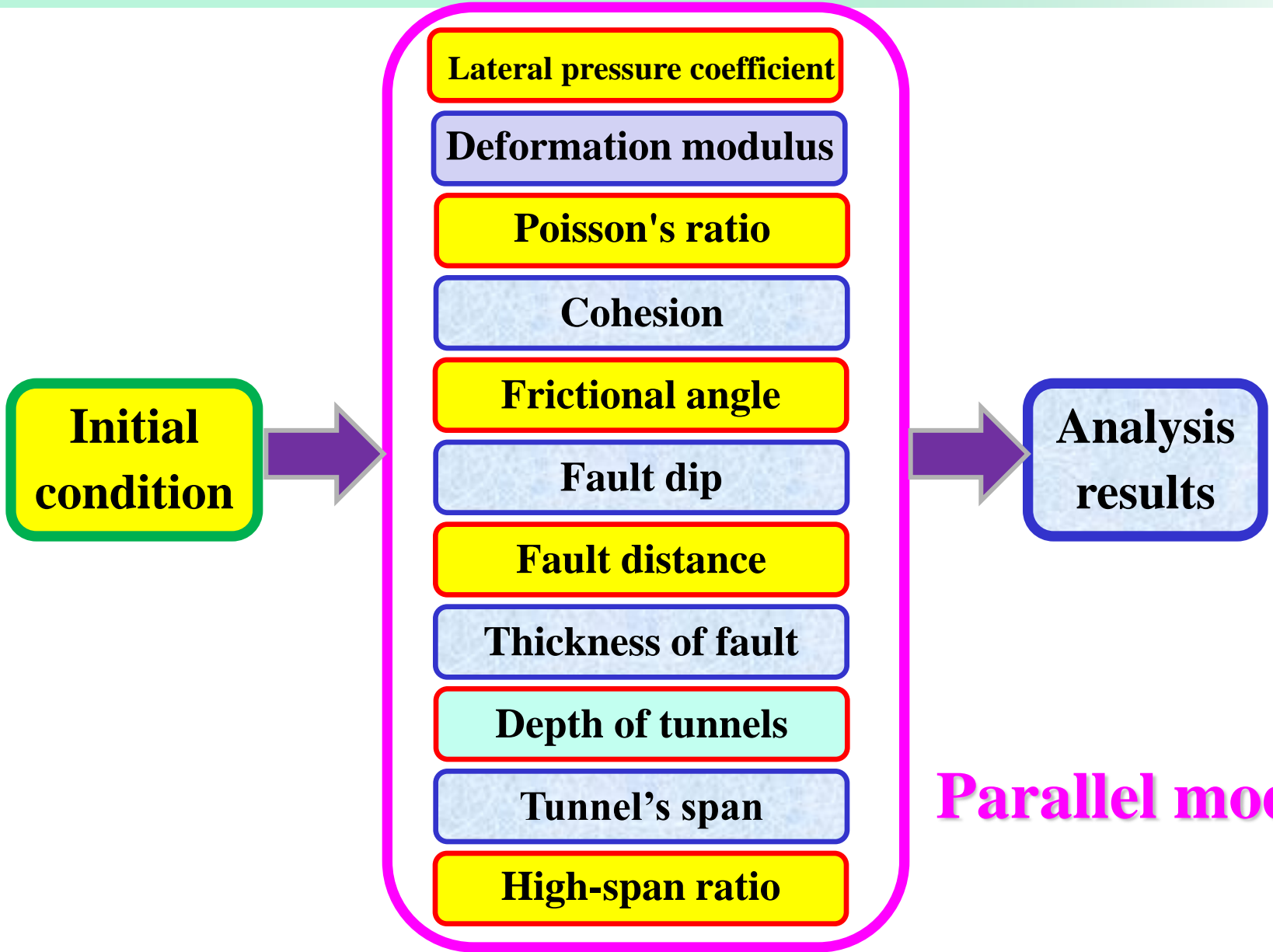
# 3 Building the FAST—Numerical tests

**Tab.1 Parameters of the numerical tests**

<b>Parameters</b>	<b>Sampling range</b>	<b>Sampling site</b>
<b>Depth (m)</b>	<b>100~1000</b>	<b>100、 500、 800、 1000</b>
<b>Lateral pressure coefficient</b>	<b>0.38~3.00</b>	<b>0.38、 1.00、 1.50、 2.00、 3.00</b>
<b>Deformation modules(Gpa)</b>	<b>0.5~20.0</b>	<b>0.5、 2.0、 8.0、 20.0</b>
<b>Poisson's ratio</b>	<b>0.22~0.32</b>	<b>0.22、 0.28、 0.32、 0.38</b>
<b>Cohesion (MPa)</b>	<b>0.15~1.8</b>	<b>0.15、 0.4、 0.8、 1.8</b>
<b>Internal frictional angle (° )</b>	<b>25~45</b>	<b>25、 30、 40、 45</b>
<b>Fault dip (° )</b>	<b>0~90</b>	<b>0、 45、 90</b>
<b>Fault distance (diameter)</b>	<b>0.2~1.0</b>	<b>0.2、 0.5、 1.0</b>
<b>Thickness of fault (cm)</b>	<b>0~200</b>	<b>0、 10、 20、 50、 200</b>
<b>Tunnel's span (m)</b>	<b>5~20</b>	<b>6、 8、 10、 15</b>
<b>High-span ratio</b>	<b>0.5~2.0</b>	<b>0.6、 1.0、 1.5、 2.0</b>



# 3 Building the FAST—Numerical tests

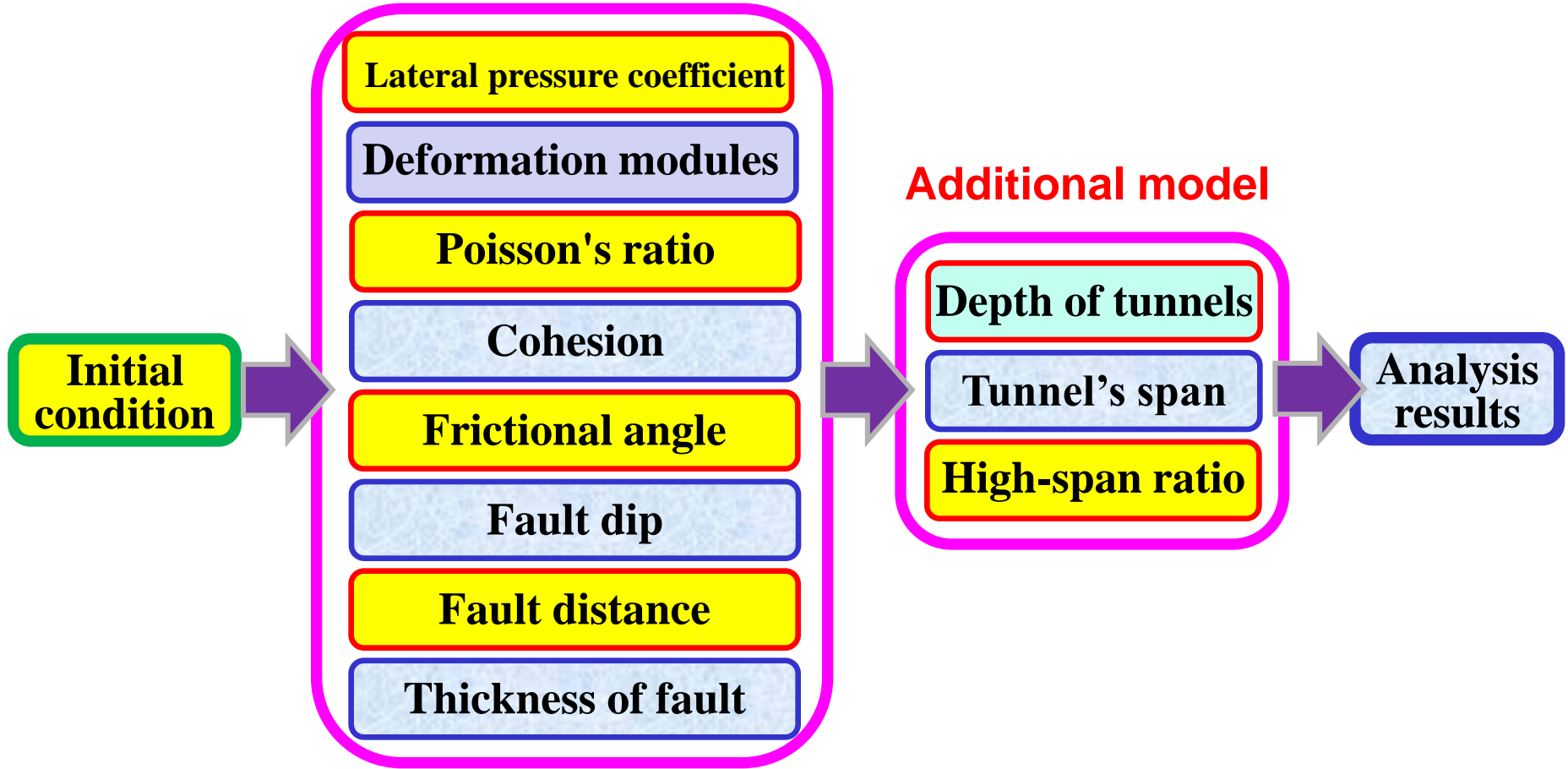






# 3 Building the FAST—Numerical tests

## The core model



**classify serial model**



# 3 Building the FAST—Numerical tests

➤ Test scheme design (about 260,000 image tunnels)

## Uniform experimental scheme design

Scheme	E	$\mu$	C	$\Phi$	K	$\alpha_1$	D <sub>1</sub>	$\alpha_2$	D <sub>2</sub>
1	20	0.2	0.4	50	1	90	0.2D	90	0.2D
2	0.5	0.28	1.8	25	2.5	180	1D	135	0.5D
3	20	0.2	1.8	40	2.5	0	0.2D	180	0.5D
4	20	0.38	0.15	25	0.38	45	1D	45	1D
5	2	0.2	0.8	50	1	45	0.2D	45	0.5D
6	8	0.2	0.8	40	2.5	135	0.5D	135	0.5D
7	2	0.28	0.8	40	1	180	0.2D	90	0.2D
8	20	0.32	1.8	50	1.5	180	0.2D	45	1D
9	2	0.2	0.4	40	1	180	0.2D	90	0.2D
10	2	0.2	0.8	50	1.5	45	1D	90	0.2D
11	0.5	0.32	0.15	50	1.5	90	0.2D	90	1D
12	20	0.32	0.15	25	2.5	135	0.5D	135	1D



# 3 Building the FAST—Numerical tests

## ➤ Test scheme design

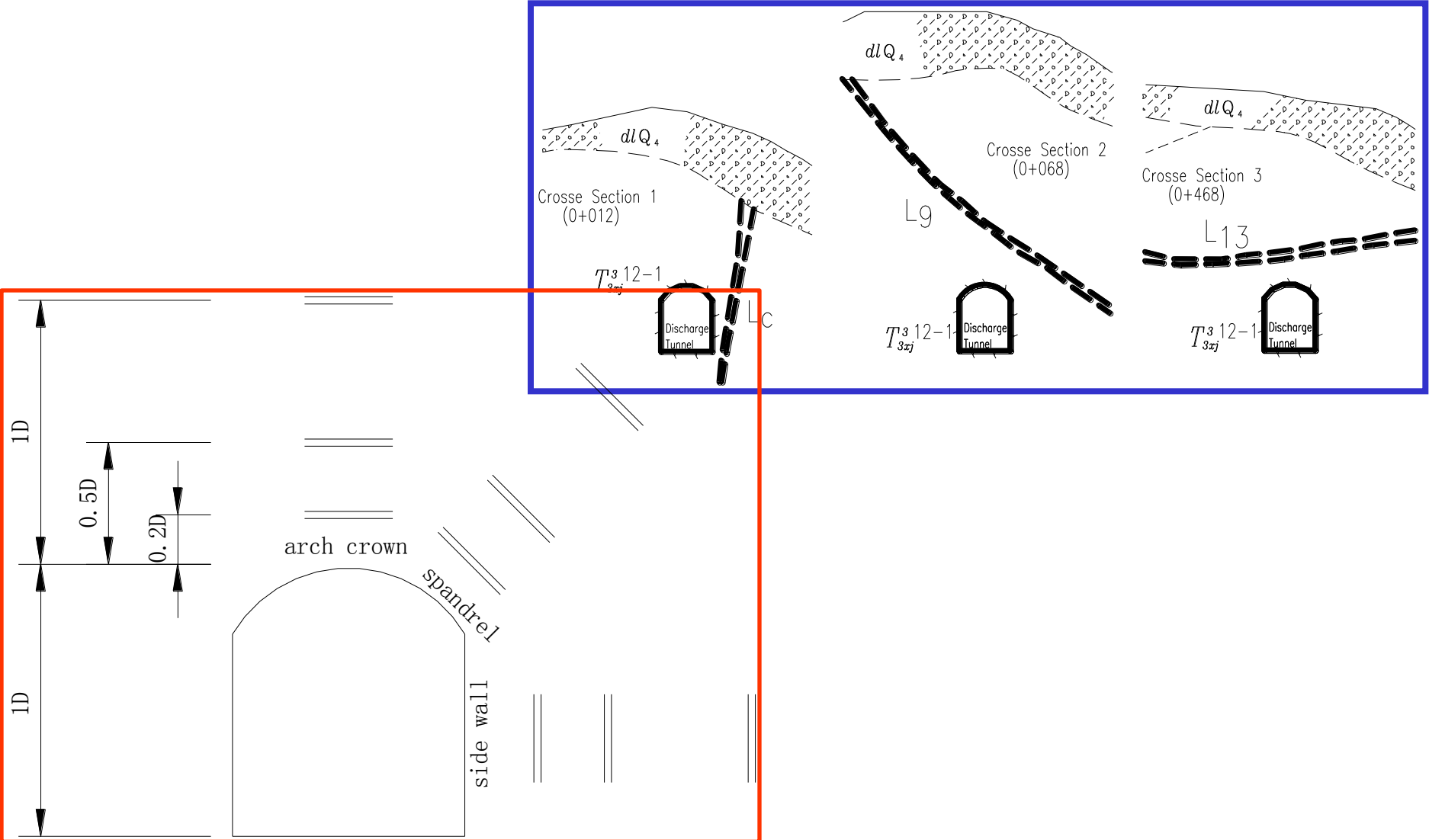
### Verification scheme design

Scheme	E(GPa)	$\mu$	C(MPa)	$\Phi$ (°)	$K_0$	$\alpha_1$ (°)	$D_1$	$\alpha_2$ (°)	$D_2$
1	8	0.32	1.8	30	0.38	0	0.2D	0	0.5D
2	2	0.2	1.8	50	1	0	1D	45	1D
3	2	0.28	0.8	40	1.5	90	1D	180	0.2D
4	0.5	0.28	0.15	40	1.5	0	0.2D	135	0.5D
5	20	0.38	0.4	40	2.5	180	1D	0	0.5D
6	8	0.32	1.8	25	1.5	45	0.2D	45	0.5D
7	8	0.2	0.4	40	1	45	1D	90	0.5D
8	0.5	0.28	0.15	30	0.38	45	1D	135	1D
9	2	0.38	0.8	25	1.5	90	0.2D	90	1D
10	20	0.32	1.8	50	1	90	0.5D	180	0.2D

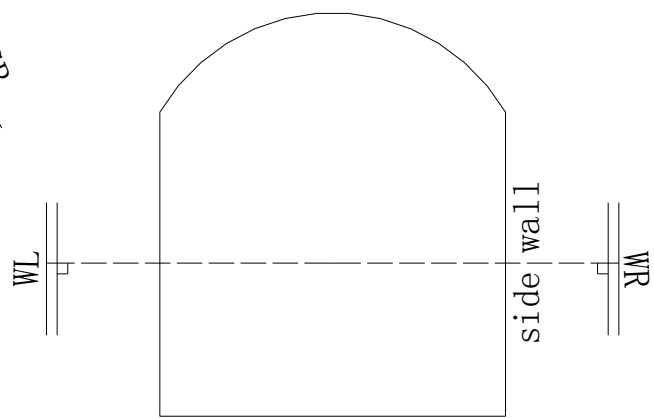
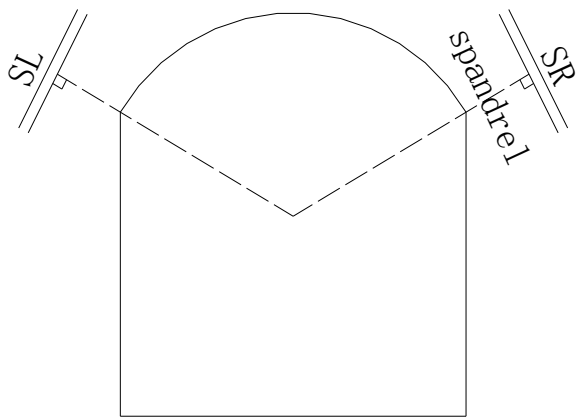
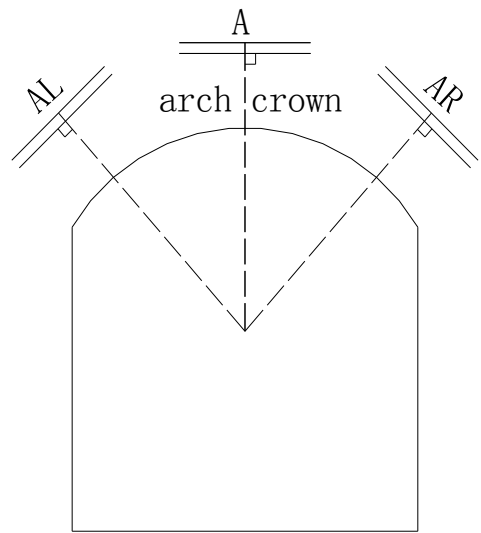


# 3 Building the FAST—Numerical tests

The modeling for the **position & distance** of faults or joints



## Composition of **two faults**

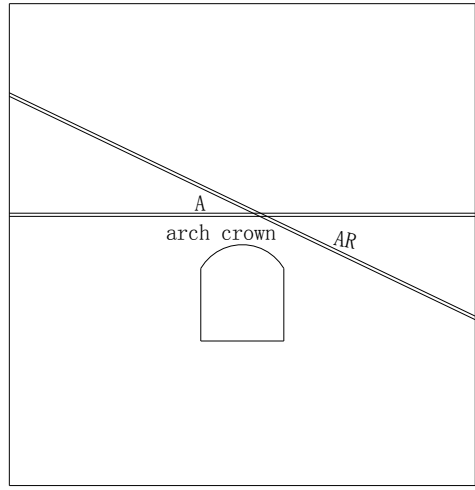




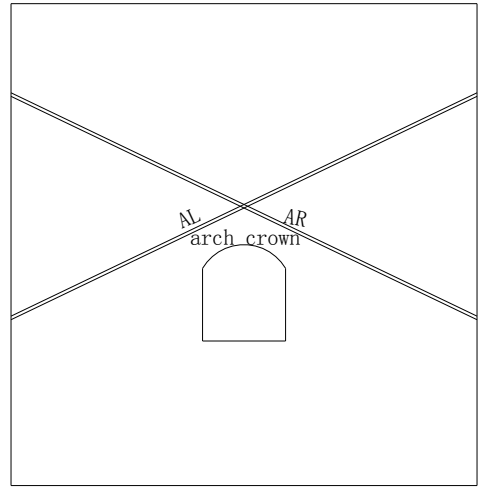
# 3 Building the FAST—Numerical tests

faults on different positions

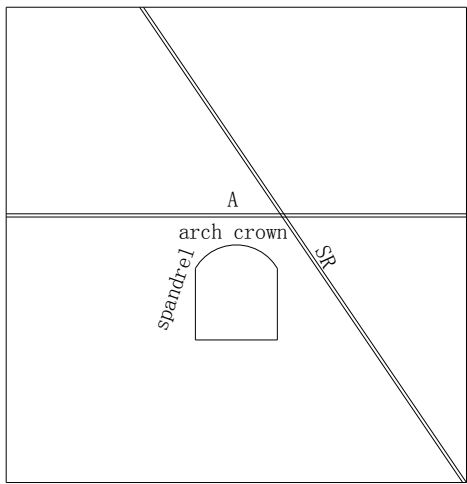
**Of the tunnel**



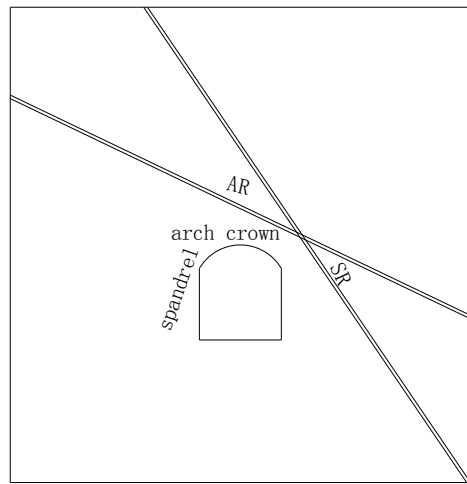
① A+AR



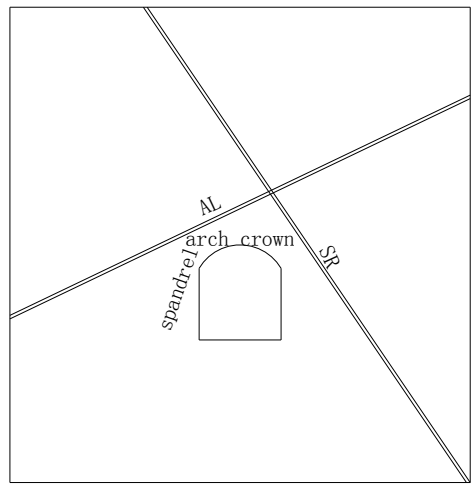
② AL+AR



① A+SR



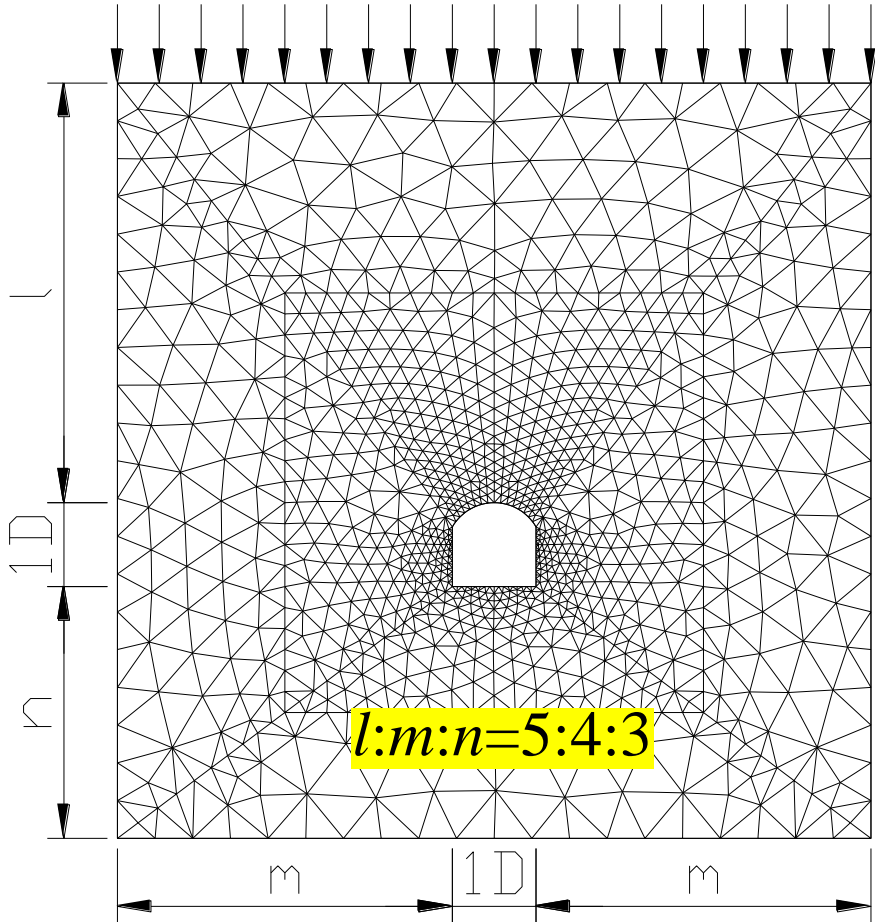
② AR+SR



③ AL+SR



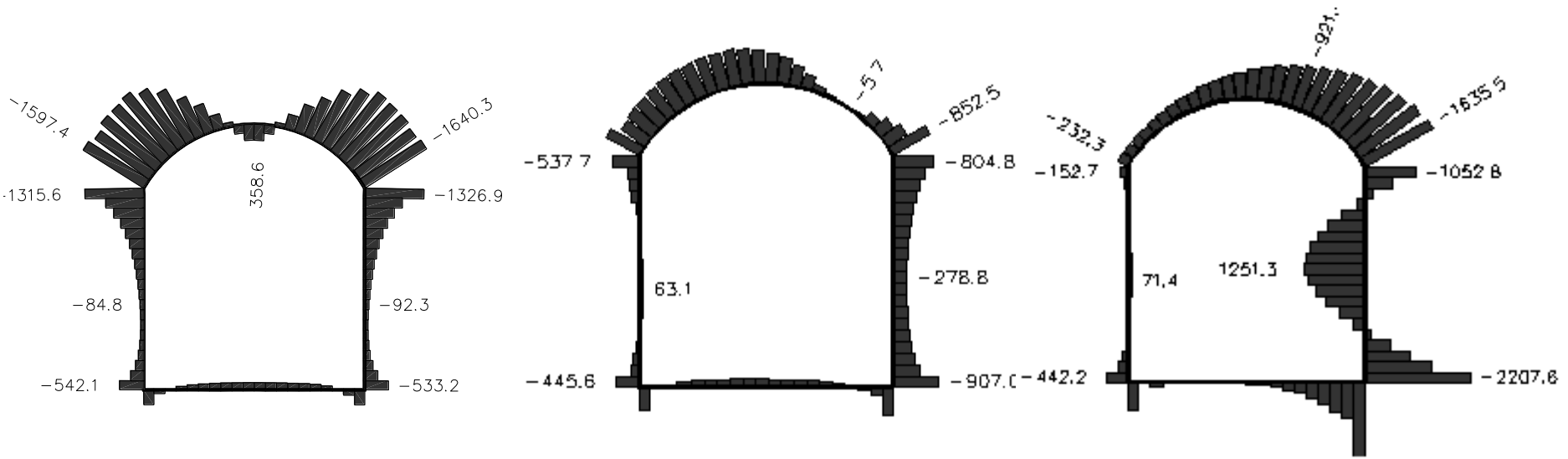
# 3 Building the FAST—Numerical tests



**The FEM model for numerical tests**

## Some results of numerical tests

(1) The impact of the **weak intercalation** distributed in **vault**, **right spandrel** and **right sidewall**.







# 3 Building the FAST—Intelligence expert system

## ➤ Supporting time

- ◆ Rock type III Stress release rate is 40% ~60%
- ◆ Rock type IV Stress release rate is 50% ~80%
- ◆ Rock type V Stress release rate is 60% ~90%



# 3 Building the FAST—Intelligence expert system

## ➤ Support scheme by Chinese code

tunnel span $B$ (m) rock type	$B \leq 5$	$5 < B \leq 10$	$10 < B \leq 15$	$15 < B \leq 20$	$20 < B \leq 25$
III	(1)80~100mm shotcrete (2)50mm shotcrete +1.5~2.0m bolt	(1)120~150mm shotcrete (2)80~100mm shotcrete +2.0~2.5m bolt (bar-mat reinforcement, in necessity)	100~150mm shotcrete +bar-mat reinforcement +3.0~4.0m bolt	150~200mm shotcrete +bar-mat einforcement +4.0~5.0m bolt (>5m prestressing force bolt or bolt, in necessity)	—
IV	80~100mm shotcrete +1.5~2.0m bolt	100~150mm shotcrete +bar-mat reinforcement +2.0~2.5m bolt (inverted arch, in necessity)	150~200mm shotcrete +bar-mat reinforcement +3.0~4.0m bolt (inverted arch+>4m bolt, in necessity)	—	—
V	120~150mm shotcrete +bar-mat reinforcement +1.5~2.0m bolt (inverted arch, in necessity)	150~200mm shotcrete+bar-mat reinforcement+2.0~3.0m bolt+inverted arch (steel arch, in necessity)	—	—	—



# 3 Building the FAST—Intelligence expert system

## ➤ Support limitations

- **bolt length** should be less than tunnel's diameter.
- **bolt length** should be no less than 1.0m.
- **bolt interval** should be less than one third of tunnel's diameter or one third of bolt length.
- **bolt interval** should be no less than 1.0m or one-tenth of tunnel's diameter.
- ... ..

## ➤ Treating of the weak layer (WL)

Decomposing the effect of **weak intercalated layer** into the effect of **crack** and the effect of its **affected zone**:



The deformation of WIL can be departed into two part:

- ✓ The discontinuous deformation caused by the **slip or open of internal cracks**.
- ✓ The continuous deformation caused by the **filling of WL**.

## ➤ Treating of the Weak Layer (WL)

The **strength of WL** element can be regarded as the **filling's strength** and the effect of WIL's **affected zone** mainly reflected the deformation characteristics of WIL whose parameters are usually referred to the **geotechnical parameters** in no-WIL zone.

$$E_z = \frac{T_z(E_r E_f)}{(T_z - T_f)E_f + T_f E_r}$$

Where,  $T_z$  is the thickness of WIL's affected zone ;

$T_f$  is the thickness of WIL;

$E_f$  is the deformation modulus of WIL;

$E_r$  is the deformation modulus of no-WIL rock.



## 3 Building the FAST—Intelligence expert system

### ➤ Fault impact factor

The **fault impact factor** is the **deformation ratio** of the same key points for two tunnels which are almost the same but **one with fault and the other without fault.**



# 3. Building the FAST—Neural network modeling

## ➤ Sample preparation

### ● Unified disposal for the **inputs**

Let the input bits of sample is  $x_p$  ( $p=1, 2, \dots, n$ ), define  $x_{\max} = \max\{x_p\}$ ,  $x_{\min} = \min\{x_p\}$ , then the inputs could be disposed by the following formula:

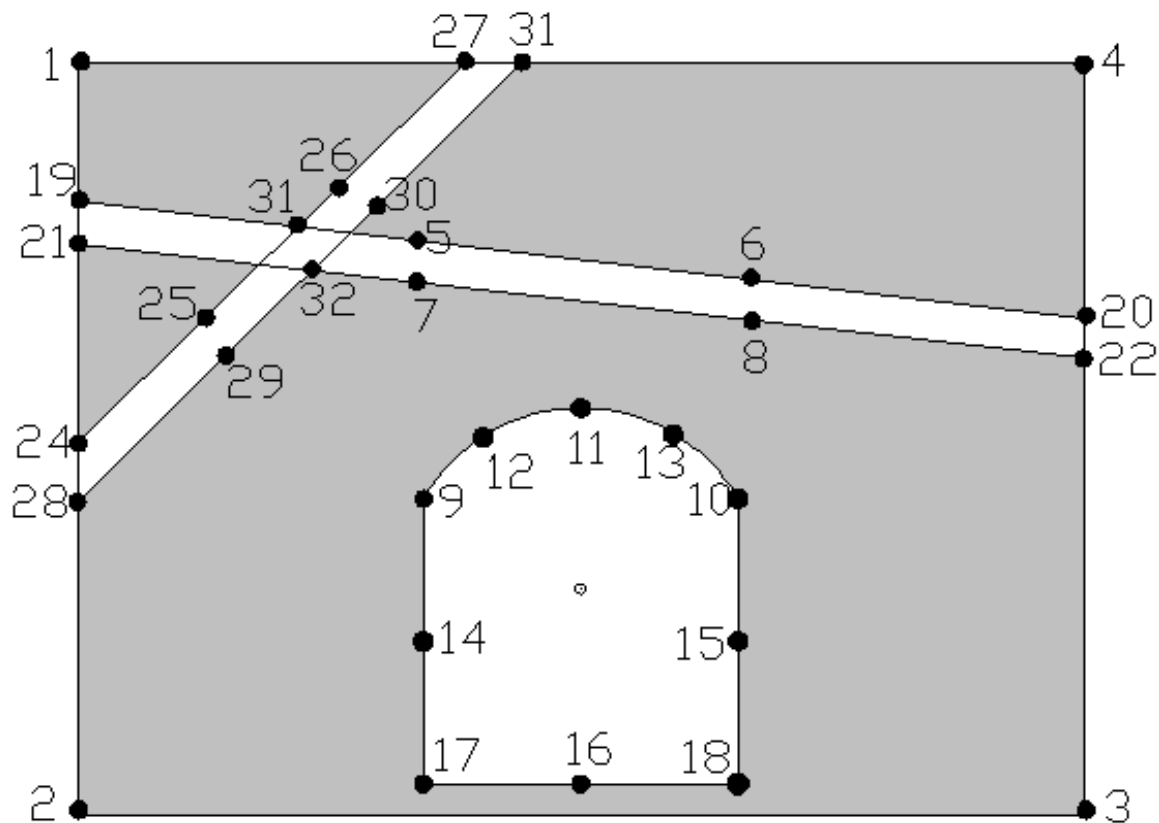
$$\frac{x_p - x_{\min}}{x_{\max} - x_{\min}} \Rightarrow x_p$$

### ● Unified disposal for the **outputs**

The outputs of the sample is  $y$ , considering its particularity, it could be disposed by the following formula:

$$\frac{\lg y}{5} \Rightarrow y$$

## ➤ Network building



**32 keypoints × 5 variables = 160 outputs**



## ➤ Training network



Have continuously calculated 6 months with 6 high speed computers.



# 3Building the FAST—Neural network modeling

## ➤ Network Testing

The training results when the fault dip is 0°

Scheme	Fault distance/D	Fault thickness/cm	Modulus /MPa	Cohesion /MPa	Friction angle /°	Poisson's ratio	Lateral pressure ratio	Accuracy
1	1	0	3000	1.8	40	0.22	3	90%
2	1	20	6000	0.8	28	0.28	1.5	89%
3	0.5	20	3000	1.8	28	0.32	0.38	87%
4	1	0	15000	1.2	28	0.28	3	91%
5	0.2	20	3000	1.2	50	0.22	0.38	93%
6	0.2	200	3000	1.8	50	0.32	1.5	88%
7	0.2	0	15000	1.8	50	0.28	0.38	89%
8	0.5	200	15000	1.8	40	0.32	3	92%
9	1	0	15000	1.2	50	0.22	1.5	90%
10	1	200	15000	0.8	40	0.28	3	89%
11	1	20	3000	0.8	28	0.32	3	88%
12	1	200	6000	1.2	50	0.32	3	91%
13	1	20	6000	0.8	40	0.22	0.38	88%
14	0.5	0	3000	1.2	28	0.32	1.5	94%
15	0.2	0	6000	1.2	40	0.32	0.38	92%
16	0.5	0	15000	0.8	28	0.28	0.38	89%
17	0.2	0	3000	0.8	40	0.22	1.5	87%
18	0.2	200	3000	0.8	40	0.28	0.38	87%



# 3. Building the FAST—Neural network modeling

## ➤ Network Testing

The training results when the fault dip is 40°

Scheme	Fault distance/D	Fault thickness/cm	Modulus /MPa	Cohesion /MPa	Friction angle /°	Poisson's ratio	Lateral pressure ratio	Accuracy
1	1	0	3000	1.8	40	0.22	3	91%
2	1	20	6000	0.8	28	0.28	1.5	87%
3	0.5	20	3000	1.8	28	0.32	0.38	86%
4	1	0	15000	1.2	28	0.28	3	91%
5	0.2	20	3000	1.2	50	0.22	0.38	90%
6	0.2	200	3000	1.8	50	0.32	1.5	86%
7	0.2	0	15000	1.8	50	0.28	0.38	88%
8	0.5	200	15000	1.8	40	0.32	3	86%
9	1	0	15000	1.2	50	0.22	1.5	91%
10	1	200	15000	0.8	40	0.28	3	87%
11	1	20	3000	0.8	28	0.32	3	87%
12	1	200	6000	1.2	50	0.32	3	90%
13	1	20	6000	0.8	40	0.22	0.38	89%
14	0.5	0	3000	1.2	28	0.32	1.5	92%
15	0.2	0	6000	1.2	40	0.32	0.38	91%
16	0.5	0	15000	0.8	28	0.28	0.38	92%
17	0.2	0	3000	0.8	40	0.22	1.5	88%



# 3. Building the FAST—Neural network model

## ➤ Network Testing

The training results when the fault dip is 90°

Scheme	Fault distance/D	Fault thickness/cm	Modulus /MPa	Cohesion /MPa	Friction angle /°	Poisson's ratio	Lateral pressure ratio	Accuracy
1	1	0	3000	1.8	40	0.22	3	89%
2	1	20	6000	0.8	28	0.28	1.5	88%
3	0.5	20	3000	1.8	28	0.32	0.38	88%
4	1	0	15000	1.2	28	0.28	3	89%
5	0.2	20	3000	1.2	50	0.22	0.38	88%
6	0.2	200	3000	1.8	50	0.32	1.5	87%
7	0.2	0	15000	1.8	50	0.28	0.38	90%
8	0.5	200	15000	1.8	40	0.32	3	86%
9	1	0	15000	1.2	50	0.22	1.5	89%
10	1	200	15000	0.8	40	0.28	3	88%
11	1	20	3000	0.8	28	0.32	3	89%
12	1	200	6000	1.2	50	0.32	3	88%
13	1	20	6000	0.8	40	0.22	0.38	87%
14	0.5	0	3000	1.2	28	0.32	1.5	91%
15	0.2	0	6000	1.2	40	0.32	0.38	91%
16	0.5	0	15000	0.8	28	0.28	0.38	90%



# 3 Building the FAST—Stability Assessment

## (1) By Chinese Code

The national **standard** of “Specifications for bolt-shotcrete (GB50086-21)” has **specified** the **deformation allowable** ranges for the tunnels in different rock type and depth.

Rock type	Depth (m)		
	<50	50~300	300~500
III	0.1-0.3	0.2-0.5	0.4-1.2
IV	0.15-0.5	0.4-1.2	0.8-2.0
V	0.2-0.8	0.6-1.6	1.0-3.0

**Note:** the data in table is the **percentage** of allowable deformation and tunnel’s diameter.

## (2) Method based on the improved Finer formula

① The maximum deformation of arch crown when the **plastic zone radius is zero**

$$\delta_{Allowable} = r \sin \phi (\gamma H + c \cdot \text{ctg} \phi) / 2G$$

② The maximum deformation of arch crown when the **plastic zone radius is R**

$$\delta_{Allowable} = r(1 - \sqrt{1 - B})$$

$$B = K_1 K_2 K_3$$

$$K_1 = 2 - \frac{1+u}{E} \sin \phi (\gamma H + C \text{ctg} \phi) \quad K_2 = \frac{1+u}{E} \sin \phi (\gamma H + C \text{ctg} \phi) \quad K_3 = \left(\frac{R}{r}\right)^2$$



# 3 Building the FAST—Operation interface



Xi'an University of Technology

The Numerical Simulation Center of Geotechnical Engineering

Fast Analysis System for Tunneling

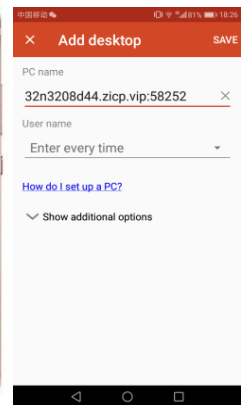
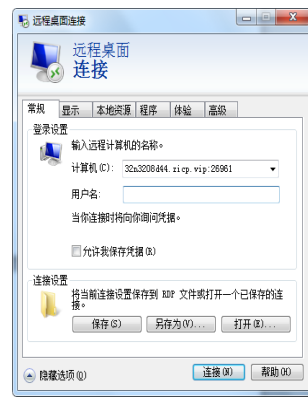
Field Fast Intelligence



ver:2.0 time:2012



# 3 Building the FAST—Operation interface



**Computer remote login**

**Mobile phone remote desktop**

**Computer users:** Direct login the remote desktop of windows system

**Phone users** : Need to install the APP, Microsoft Remote Desktop

PC name: 32n3208d44.zicp.vip:26961 or 58252

User name: blank







# 4 Verification of FAST



The diversion tunnel of Huang river **Jishixia hydropower station**.  
Weak intercalation(Pjn33) through tunnel roof.

Key points	Results by FINAL	Results by FAST	Relative error
Arch crown	-26.12mm	-25.45mm	3%
Mid leftwall	2.58mm	2.89mm	12%
Mid rightwall	-1.31mm	-1.44mm	10%

# 4 Verification of FAST



The diversion tunnel of **Zipingbu hydraulic project**.  
Weak intercalation(L9, F3) through tunnel.

Key points	Measured results	Results by FAST	Relative error
Arch crown	-24.13mm	-26.62mm	9%
Mid leftwall	35.01mm	30.51mm	13%
Mid rightwall	-13.59mm	-16.03mm	18%



**Thank you for your attention!**